

Example 9-19:

The cross section is composed of two channels back to back to form an H-section. One C-shape has the following parameters:

$A = 8.82 \text{ in}^2$, $I_x = 103 \text{ in}^4$, $I_y = 3.94 \text{ in}^4$, $x_c = 0.649 \text{ in}$ (distance from outer face of web to centroid), $E = 29,000 \text{ ksi}$, $\sigma_y = 36 \text{ ksi}$.

- Where will the truss fail?
- How and under which load will it fail when the factor of safety for yield is 1.75 and for buckling is 4?

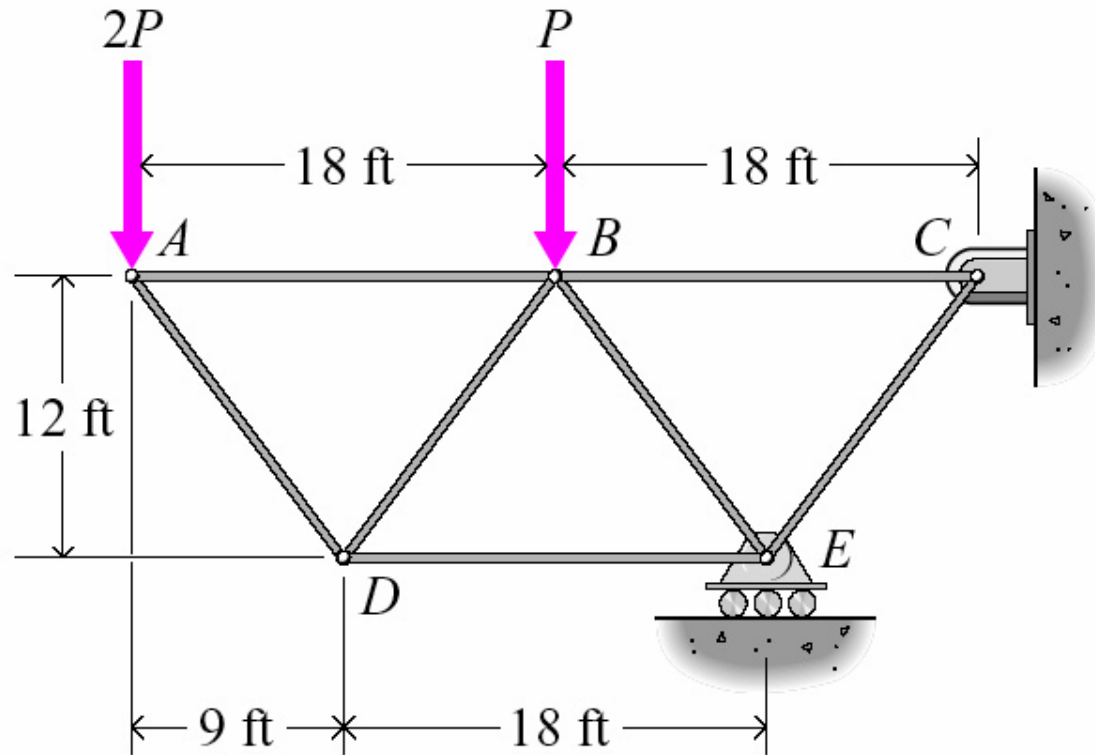
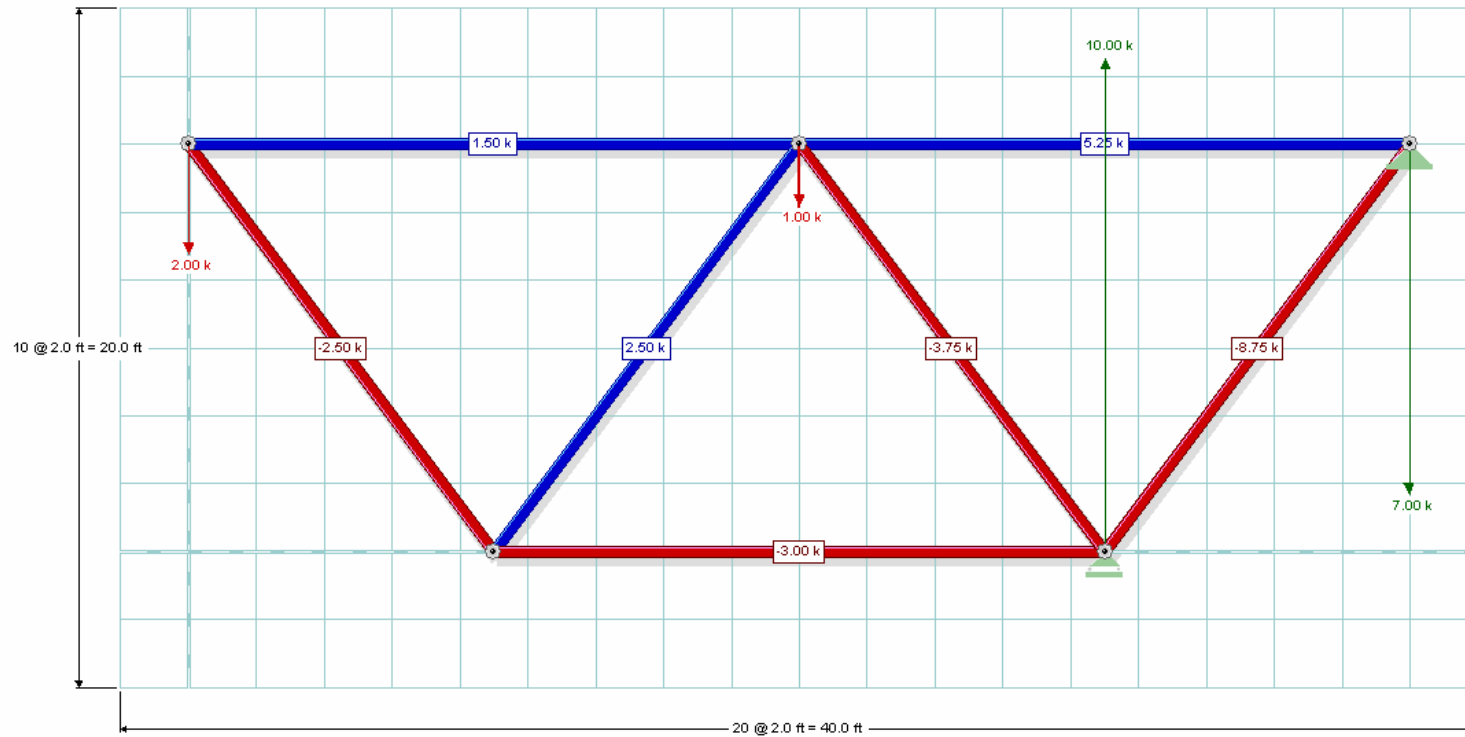


Figure P9-19

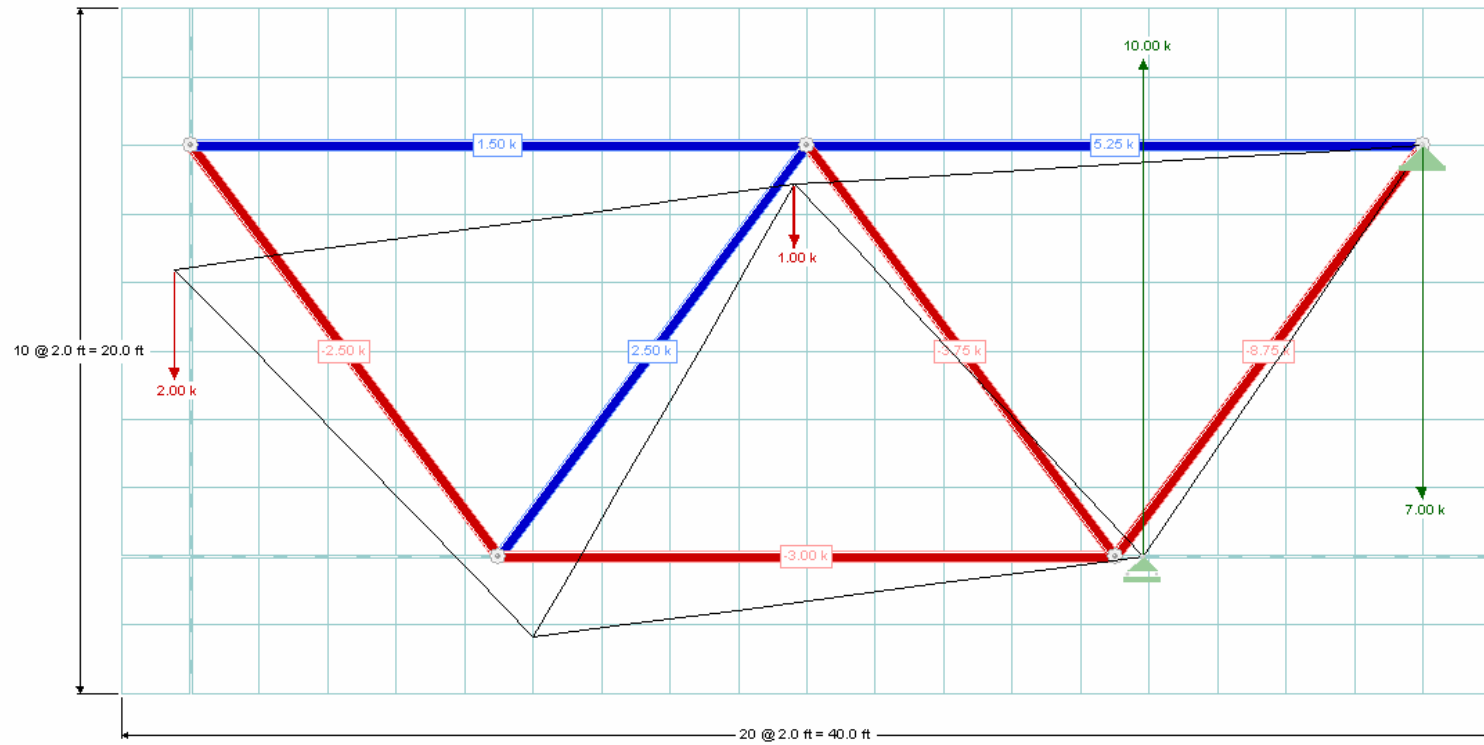
member forces computed with DrFrame:

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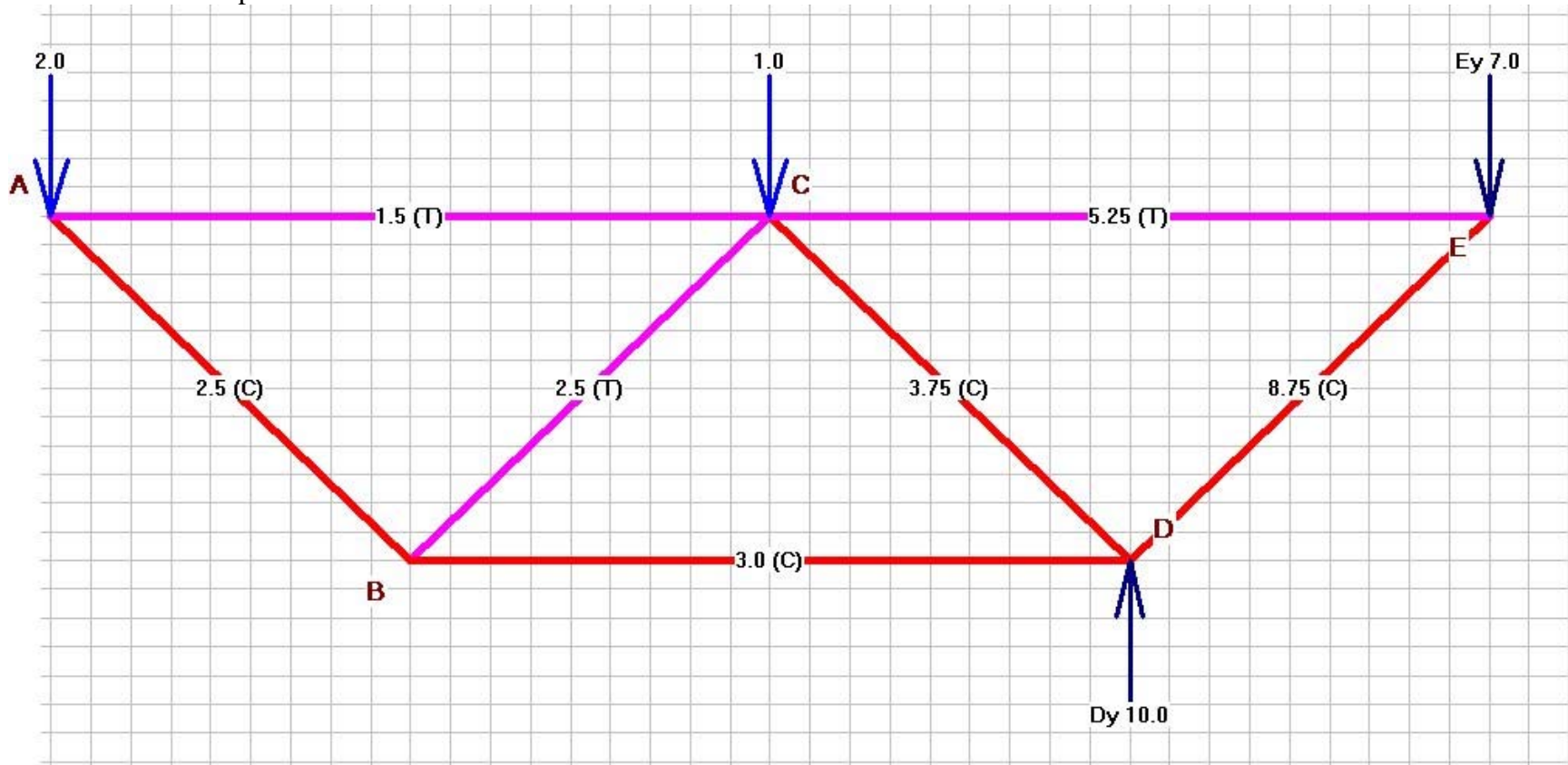


deflections computed with DrFrame:

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member forces computed with MDSolids:



Textbook solution see next page (long hand computation with calculator):

9-19

From the overall free-body diagram:

$$\circlearrowleft \Sigma M_C = 0: \quad 2P(36) + P(18) - R_E(9) = 0$$

$$R_E = 10P$$

From a free-body diagram of pin A:

$$\theta = \tan^{-1}(12/9) = 53.13^\circ$$

$$\uparrow \Sigma F_y = 0: \quad -T_{AD} \sin 53.13^\circ - 2P = 0$$

$$T_{AD} = -2.50P = 2.50P \text{ (C)}$$

$$\rightarrow \Sigma F_x = 0: \quad T_{AB} + T_{AD} \cos 53.13^\circ = 0$$

$$T_{AB} = +1.50P = 1.50P \text{ (T)}$$

From a free-body diagram of the part of the truss to the left of pin B:

$$\uparrow \Sigma F_y = 0: \quad T_{BD} \sin 53.13^\circ - 2P = 0$$

$$T_{BD} = +2.50P = 2.50P \text{ (T)}$$

From a free-body diagram of the part of the truss to the left of pin E:

$$\uparrow \Sigma F_y = 0: \quad -T_{BE} \sin 53.13^\circ - 3P = 0$$

$$T_{BE} = -3.75P = 3.75P \text{ (C)}$$

$$\circlearrowleft \Sigma M_B = 0: \quad T_{DE}(12) + 2P(18) = 0$$

$$T_{DE} = -3.00P = 3.00P \text{ (C)}$$

From a free-body diagram of the part of the truss to the left of pin C:

$$\uparrow \Sigma F_y = 0: \quad T_{CE} \sin 53.13^\circ + 10P - 3P = 0$$

$$T_{CE} = -8.75P = 8.75P \text{ (C)}$$

$$\circlearrowleft \Sigma M_E = 0: \quad 2P(27) + P(9) - T_{BC}(12) = 0$$

$$T_{BC} = +5.25P = 5.25P \text{ (T)}$$

From Table B-5 for a C 10×30 section:

$$A = 8.82 \text{ in.}^2$$

$$x_C = 0.649 \text{ in.}$$

$$I_x = 103 \text{ in.}^4$$

$$I_y = 3.94 \text{ in.}^4$$

From Table B-17:

$$E = 29,000 \text{ ksi}$$

$$\sigma_y = 36 \text{ ksi}$$

For member BC:

$$F_{\max} = \sigma_y A = (36)(2 \times 8.82) = 635.0 \text{ kip}$$

$$T_{BC} = 5.25P \leq \frac{F_{\max}}{FS} = \frac{635.0}{1.75}$$

$$P \leq 69.1 \text{ kip}$$

For the bolted channels:

$$I_y = I_{\min} = 2 \left[3.94 + 8.82(0.649)^2 \right] = 15.31 \text{ in.}^4$$

For member CE:

$$P_{cr} = \frac{\pi^2 EI}{L^2} = \frac{\pi^2 (29,000)(15.31)}{(15 \times 12)^2} = 135.25 \text{ kip}$$

$$T_{CE} = 8.75P \leq \frac{P_{cr}}{FS} = \frac{135.25}{4}$$

$$P \leq 3.86 \text{ kip}$$

Therefore

$$P_{\max} = 3.86 \text{ kip} \text{ Ans.}$$

